FOUNDATION PLAN

1/4" = 1'-0"

ELEVATION TOP OF FINISHED FLOOR SLAB TO MATCH EXIST. (72.53' V.I.F.)
EL. 0'-0" UNLESS OTHERWISE NOTED ON PLAN.
NEW SLAB IN ADDITION: 6" CONC. SLAB + 6x6-W2.9xW2.9 W.W.F. ON A 15 MIL VAPOR BARRIER OVER 6" MIN. DRAINAGE FILL UNLESS OTHERWISE NOTED.
ELEVATION BOTTOM OF FOOTINGS NOTED THUS ( ) ON PLAN BELOW FINISHED FLOOR DATUM EL. 0'-0".
C.J. - INDICATES SLAB CONSTRUCTION OR CONTROL JOINT.
SLAB DEPRESSIONS NOTED THUS [ ] ON PLAN FROM TOP OF FLOOR SLAB - SEE ARCH. DWGS. FOR LOCATION & EXTENT.
{B. PL. } - INDICATES BOTTOM OF BASE PLATE ABOVE OR BELOW DATUM.
ANY PENETRATIONS IN SLAB FROM ALL TRADES TO BE FILLED WITH POLYURETHANE CAULK.

NOTE:
1. STEP ALL FOOTINGS AT ALL PIPES & CONDUITS, SEE TYPICAL DETAIL.

2. G.C. TO CONFIRM ALL LOCATIONS AND INVERTS OF PIPES WITH MECHANICAL & ELECTRICAL CONTRACTORS.

3. THE FOUNDATION DRAWINGS SHOW ONLY GENERAL LOCATIONS AND MAY NOT INCLUDE ALL PENETRATIONS. G.C. TO SEE MECHANICAL DRAWINGS FOR FULL EXTENT OF PIPE PENETRATIONS.

NOTE:
GENERAL CONTRACTOR TO LAYOUT DIMENSIONS OF COLUMN CENTERLINES AT THE BEGINNING OF THE PROJECT AND IS TO NOTIFY THE ARCHITECT AND STEEL CONTRACTOR OF ANY DEVIATIONS FROM THE CONTRACT DRAWINGS. SEE S1.1 FOR FOOTING AND PIER SCHEDULES.

NOTE:
ELECTRICAL CONTRACTOR TO PROVIDE GROUNDING ELECTRODE SYSTEM AS REQUIRED BY NEC SECTION 250-52.a. COORDINATE LOCATION WITH GENERAL CONTRACTOR.

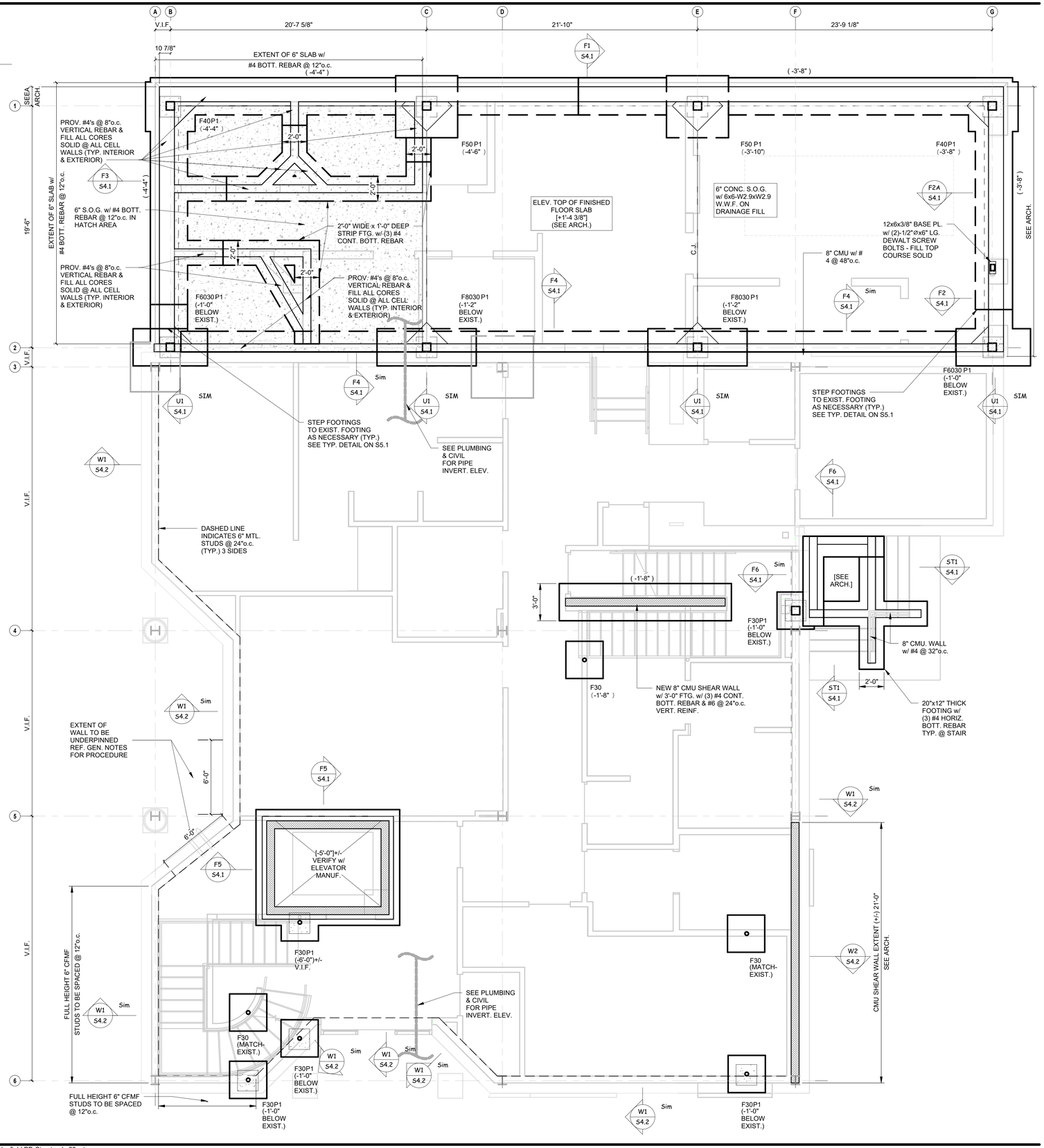
FOOTING SCHEDULE

MARK	SIZE	REINFORCEMENT
F30	3' - 0" x 3' - 0" x 12"	3 - # 4 E.W. BOT.
F40	4' - 0" x 4' - 0" x 12"	4 - # 5 E.W. BOT.
F50	5' - 0" x 5' - 0" x 14"	5 - # 6 E.W. BOT.
F6030	6' - 0" x 3' - 0" x 12"	3 - # 6 L.W. BOT. 6 - # 6 S.W. BOT.
F8030	8' - 0" x 3' - 0" x 14"	3 - # 6 L.W. BOT. 8 - # 6 S.W. BOT.

PIER SCHEDULE

MARK	SIZE	REINFORCEMENT	REMARKS
P1	20" x 20"	(6) - # 6 VERT.	

NOTES:
1. ALL PIERS TO HAVE # 3 TIES @ 12" O.C. PLUS (2) #3 TIES IN TOP 5" OF PIER (TYPICAL)
2. PROVIDE BRICKSHELF AS REQUIRED. SEE TYPICAL BRICKSHELF DETAIL ON DRAWING S5.1



CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
121 MARKET STREET
CAMDEN, NJ 08102
PHONE: (866) 974 7666

SIGNATURE:

THOMAS S. PERRINO	21A01505400
SCOTT E. DOWNIE	21A01674400
STEVEN LEONE	21A01170100
STEVEN G. SIEGEL	21A01564200
ANGELO ALBERTO	21A01046700
JOHN F. WRIGHT	21A01784200
SPIEZLE ARCHITECTURAL GROUP, INC.	21AG00063000

SEAL:

Harrison - Hamnett, P.C.
Consulting Engineers
4O Knowles Street Pennington, New Jersey O8534
Ph: 6O9-818-18O8 www.hhpcse.com

MARK W. GAFFNEY, NJPE #24GE04284100 DATE

BID SET - 06/25/2024

PROJECT:

NEW HADDONFIELD POLICE STATION

1 WALNUT STREET, HADDONFIELD, NJ 08033

FOR

CAMDEN COUNTY IMPROVEMENT AUTHORITY

2120 VOORHEES TOWN CENTER, VORHEES TOWNSHIP, NJ 08043

FOR CODE REVIEW: 02/23/24

REVISIONS:

#	REVISION NAME	DATE

FOR BID: 06/25/2024

DRAWING TITLE:

FOUNDATION PLAN

COMMISSION NUMBER:

23M014

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S1.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 24"x36"

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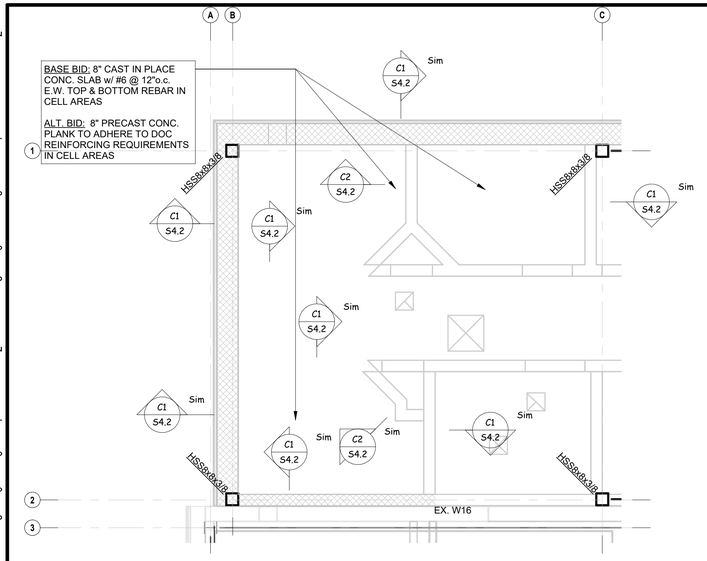
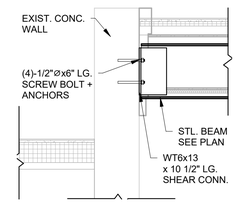


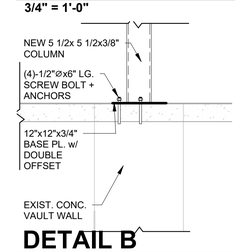
2nd FLOOR CELL CEILING PLAN

1/4" = 1'-0"



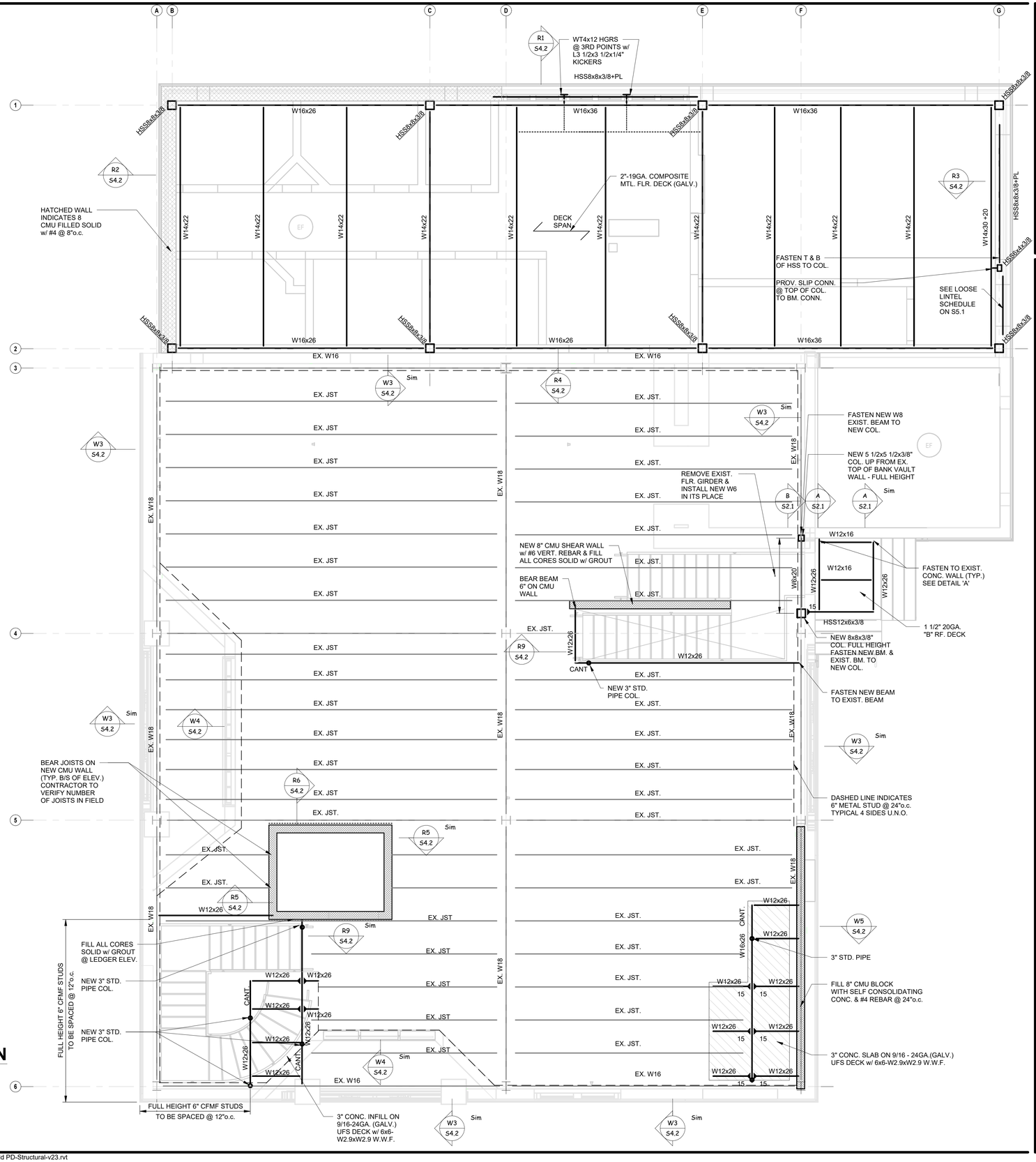
DETAIL A

3/4" = 1'-0"



DETAIL B

3/4" = 1'-0"



LOW ROOF & 2nd FLOOR FRAMING PLAN

1/4" = 1'-0"

ELEVATION TOP OF FINISHED FLOOR SLAB TO MATCH EXISTING EL. 0'-0" UNLESS OTHERWISE NOTED ON PLAN.
W 16 x 26 + 24 - INDICATES W16 x 26 STEEL BEAM WITH 24-3/4" DIA. SHEAR STUDS.
SPACE ALL STEEL MEMBERS EQUALLY UNLESS OTHERWISE NOTED ON PLAN.
◄ ► - INDICATES MOMENT CONNECTION IN UNITS OF KIP - FT.

CODE REVIEW:

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JOHN F. WRIGHT	21A01784200
SPIEZLE ARCHITECTURAL GROUP, INC.	21AZ00063000

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MARK W. GAFFNEY, NJPE #24GE04284100 DATE

BID SET - 06/25/2024

PROJECT:

NEW HADDONFIELD POLICE STATION

1 WALNUT STREET, HADDONFIELD, NJ 08033

FOR

CAMDEN COUNTY IMPROVEMENT AUTHORITY

2120 VOORHEES TOWN CENTER, VORHEES TOWNSHIP, NJ 08043

FOR CODE REVIEW: 02/23/24

REVISIONS:

#	REVISION NAME	DATE

FOR BID: 06/25/2024

DRAWING TITLE:

SECOND FLOOR FRAMING PLAN

COMMISSION NUMBER:

23M014

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S2.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 24"x36"

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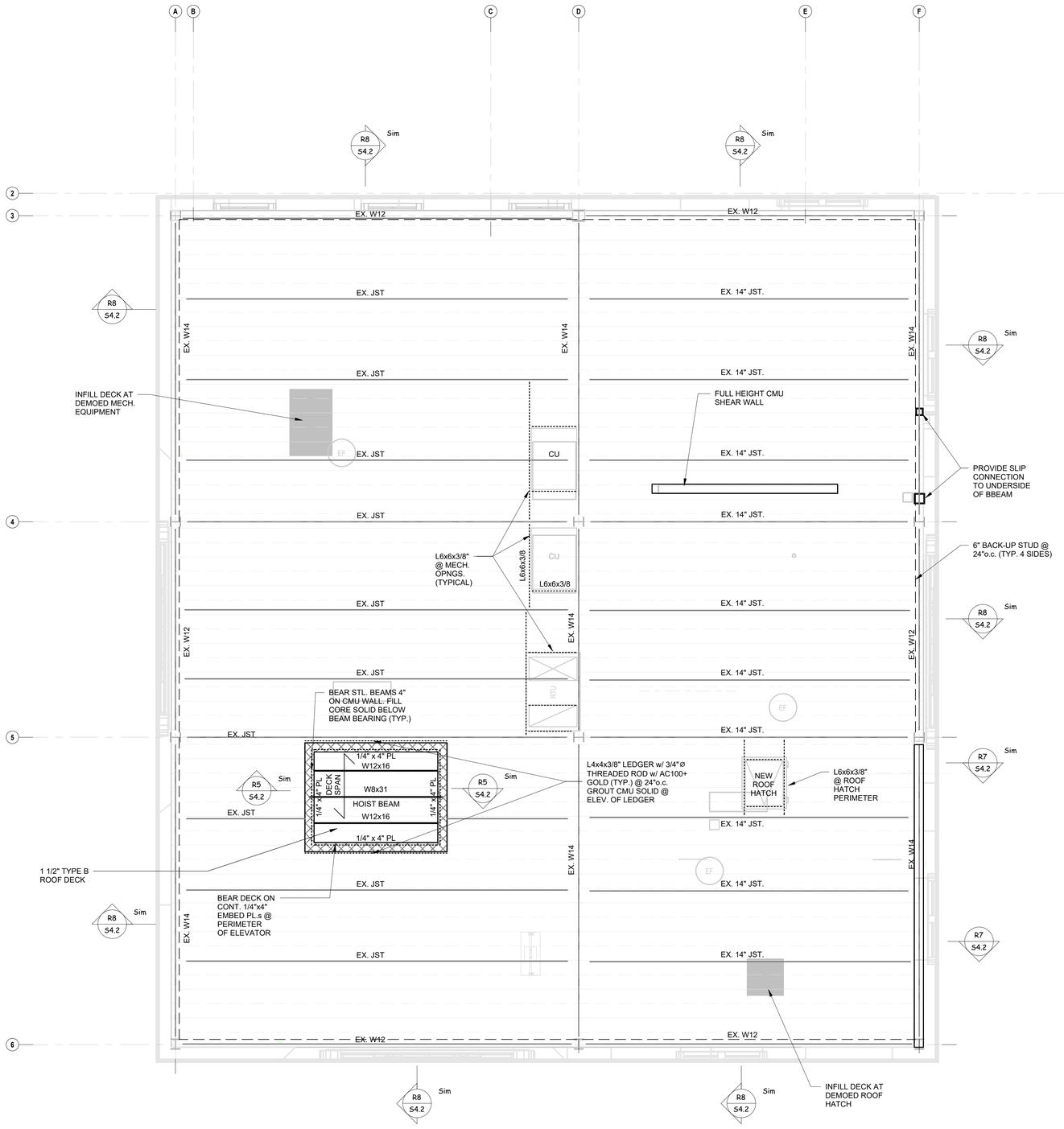

ROOF FRAMING PLAN

1/4" = 1'-0"

ROOF DECK INFILL: 1 1/2" - 20 GA. TYPE "B" (WIDE RIB) METAL ROOF DECK (GALV.).
PROVIDE FRAME AROUND ALL ROOF DRAINS & AT ALL OTHER ROOF OPENINGS LARGER THAN 8" AS PER TYPICAL DETAIL.
COORDINATE SIZE & LOCATION OF ALL ROOF OPNGS. W/ ARCH. & MECH. DWGS.
SPACE ALL STEEL MEMBERS EQUALLY UNLESS OTHERWISE NOTED ON PLAN.

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
121 MARKET STREET
CAMDEN, NJ 08102
PHONE: (866) 974 7666

SIGNATURE:

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MARK W. GAFFNEY, NJPE #24GE04284100

DATE

BID SET - 06/25/2024

PROJECT:

NEW HADDONFIELD
POLICE STATION

1 WALNUT STREET, HADDONFIELD, NJ
08033

FOR

CAMDEN COUNTY
IMPROVEMENT
AUTHORITY

2120 VOORHEES TOWN CENTER,
VORHEES TOWNSHIP, NJ 08043

FOR CODE REVIEW: 02/23/24

REVISIONS:

#	REVISION NAME	DATE

FOR BID: 06/25/2024

DRAWING TITLE:

ROOF FRAMING PLAN

COMMISSION NUMBER:

23M014

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S3.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 24"x36"

1 1/2"=1'-0" 1" 1/2" 3/4" 1/2" 1/8" 1/16"

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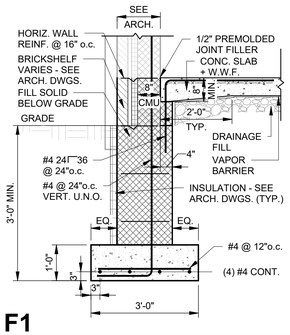
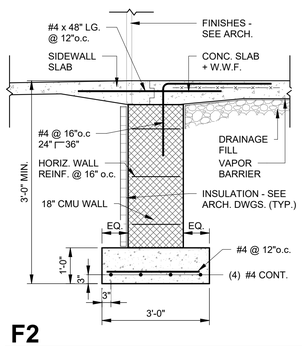


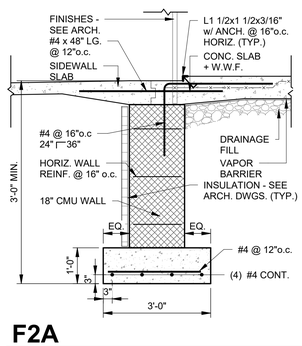
F1

1/2" = 1'-0"



F2

1/2" = 1'-0"



F2A

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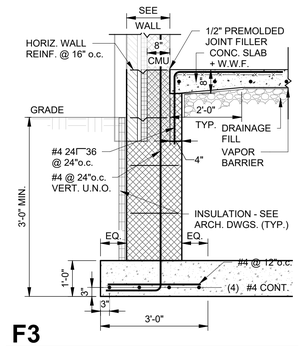

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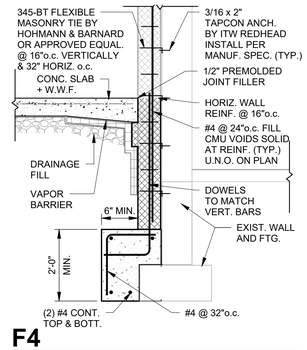
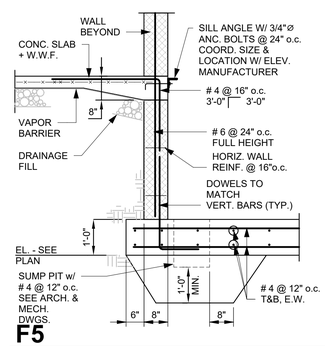


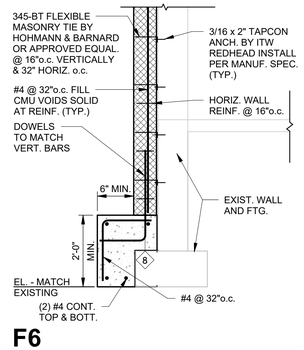
F4

1/2" = 1'-0"



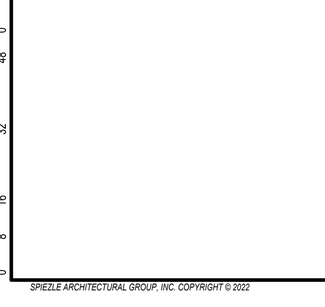
F5

1/2" = 1'-0"



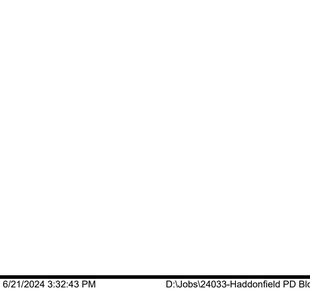
F6

1/2" = 1'-0"



F7

1/2" = 1'-0"



F8

1/2" = 1'-0"

U1

1/2" = 1'-0"

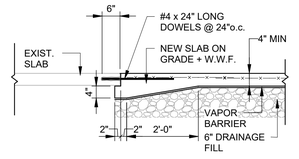
SECTION - ST1

ST1

1/2" = 1'-0"

GENERATOR FOUNDATION PLAN

1/4" = 1'-0"



TYP. SLAB INFILL DETAIL

1/2" = 1'-0"

SEE ARCH. & MEP DRAWINGS FOR LOCATION & EXTENT OF SLAB REMOVAL & INFILL

CODE REVIEW:

CERTIFICATE:

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CAMDEN, NJ 08102
PHONE: (866) 974 7666

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MARK W. GAFFNEY, NJPE #24GE04284100 DATE

BID SET - 06/25/2024

PROJECT:

NEW HADDONFIELD POLICE STATION

1 WALNUT STREET, HADDONFIELD, NJ 08033

FOR

CAMDEN COUNTY IMPROVEMENT AUTHORITY

2120 VOORHEES TOWN CENTER, VORHEES TOWNSHIP, NJ 08043

FOR CODE REVIEW: 02/23/24

REVISIONS:

#	REVISION NAME	DATE

FOR BID: 06/25/2024

DRAWING TITLE:

GENERATOR PAD PLAN & FOUNDATION SECTIONS

COMMISSION NUMBER:

23M014

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S4.1

THIS DRAWING IS FORMATTED TO BE PRINTED AT 24"X36"

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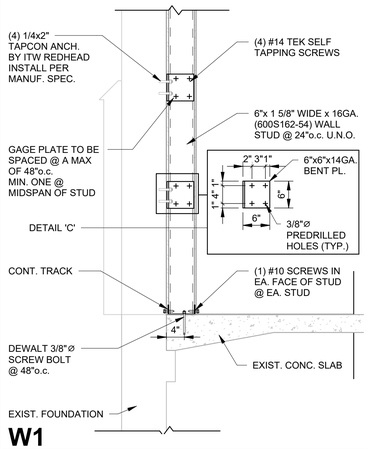
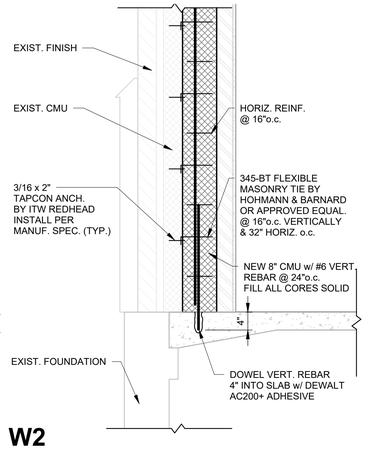


W1

3/4" = 1'-0"



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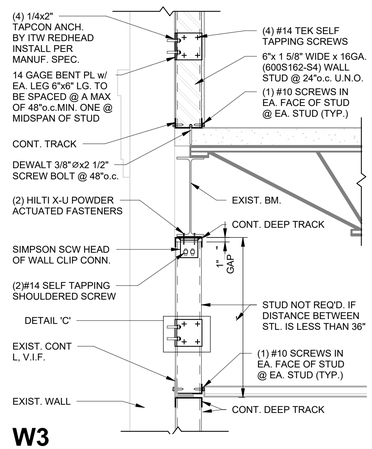

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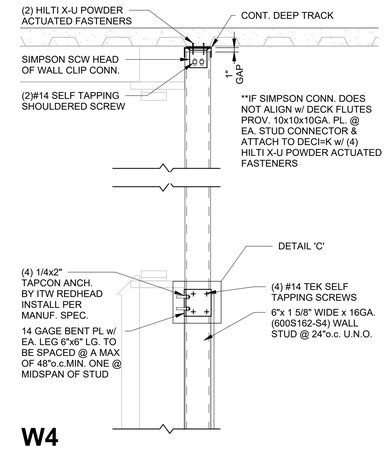

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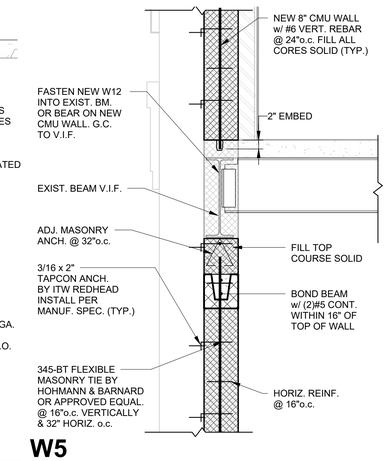
3/4" = 1'-0"



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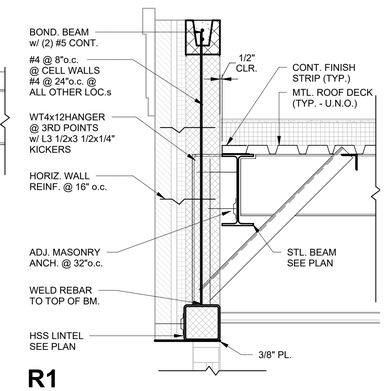
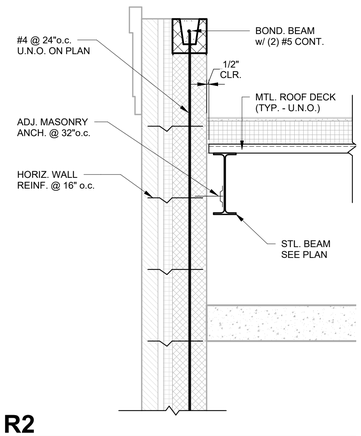


R1

3/4" = 1'-0"



R2

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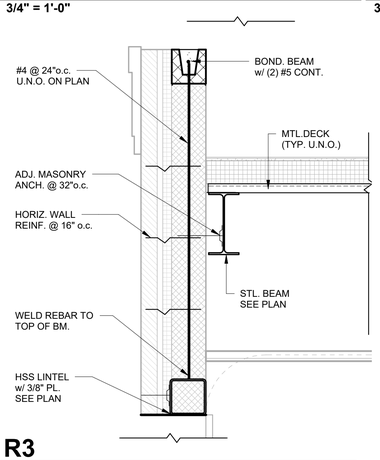

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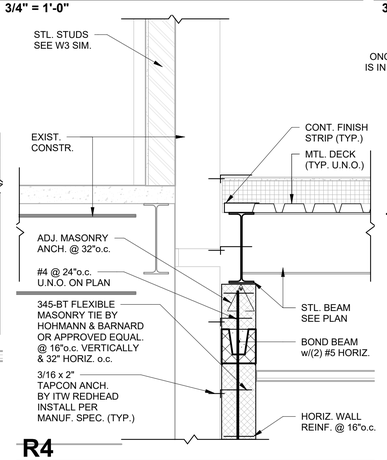

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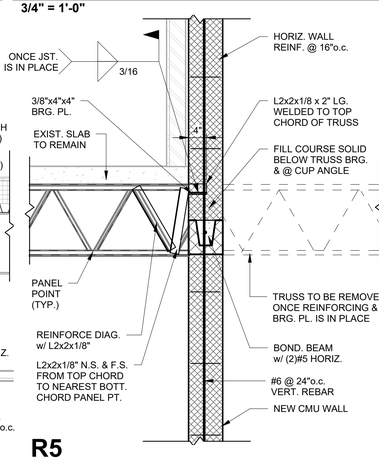

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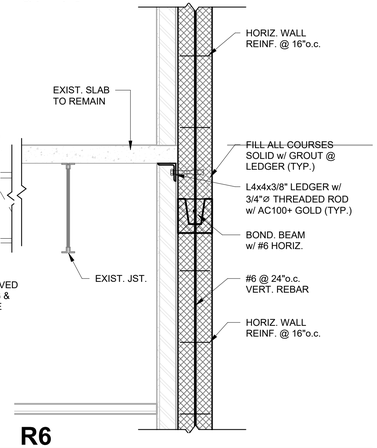

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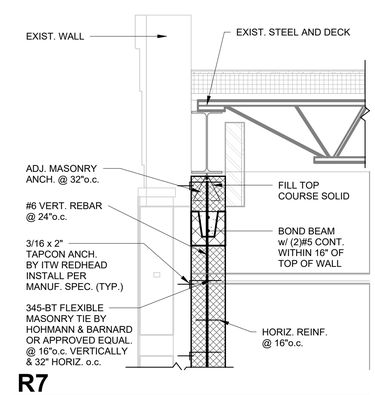
3/4" = 1'-0"



R7

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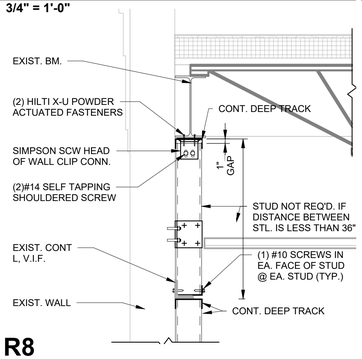

R8

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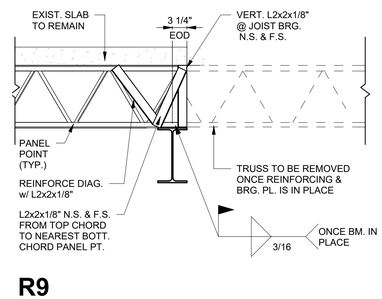

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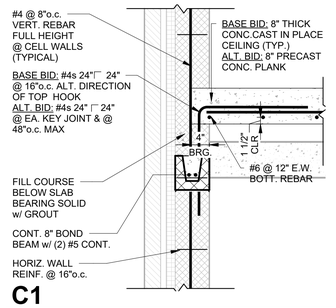
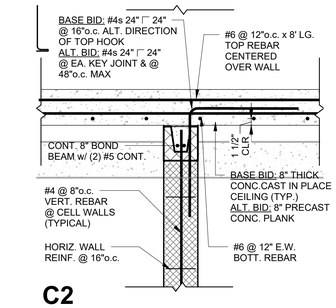


C1

3/4" = 1'-0"



C2

3/4" = 1'-0"

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SPIEZLE ARCHITECTURAL GROUP, INC.	21AZ00063000

SEAL:

Harrison - Hamnett, P.C.
Consulting Engineers
4O Knowles Street Pennington, New Jersey O8534
Ph: 6O9-818-18O8 www.hhpcse.com

MARK W. GAFFNEY, NJPE #24GE04284100 DATE

BID SET - 06/25/2024

PROJECT:

NEW HADDONFIELD POLICE STATION

1 WALNUT STREET, HADDONFIELD, NJ 08033

FOR

CAMDEN COUNTY IMPROVEMENT AUTHORITY

2120 VOORHEES TOWN CENTER, VORHEES TOWNSHIP, NJ 08043

FOR CODE REVIEW: 02/23/24

REVISIONS:

#	REVISION NAME	DATE

FOR BID: 06/25/2024

DRAWING TITLE:

SECTIONS

COMMISSION NUMBER:

23M014

DO NOT SCALE THE DRAWINGS

DRAWING NUMBER:

S4.2

THIS DRAWING IS FORMATTED TO BE PRINTED AT 24"x36"

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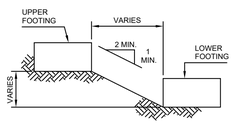
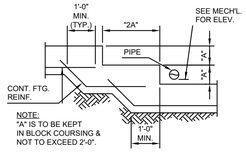
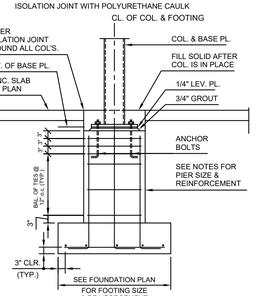


TYPICAL SLOPE BETWEEN FOOTINGS

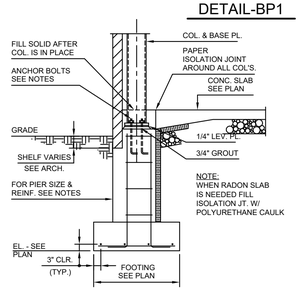


NOTE:
"A" IS TO BE KEPT IN BLOCK COURSING & NOT TO EXCEED 2'-0".

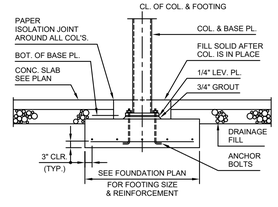
TYPICAL STEPPED FOOTING



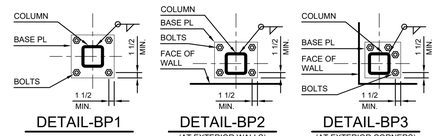
TYPICAL ISOLATED PIER & FOOTING DETAIL

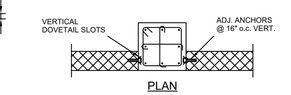


BRICK SHELF @ COLUMNS

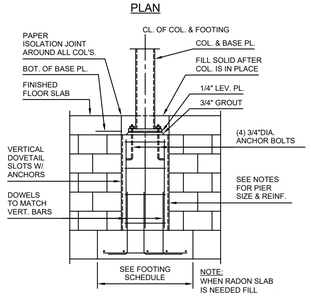


TYPICAL INTERIOR FOOTING





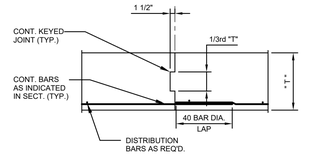
PLAN



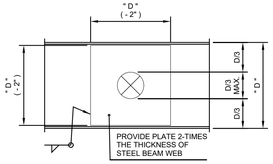
NOTE:
WHEN RADON SLAB IS NEEDED FILL ISOLATION JOINT WITH POLYURETHANE CAULK

ELEVATION

TYPICAL PIER & FOOTING DETAIL

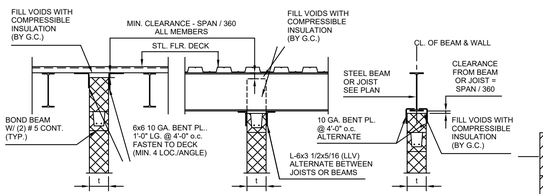


TYPICAL CONCRETE FOOTING CONSTRUCTION JOINT



NOTE:
LOCATE OPENINGS IN MIDDLE 1/3 OF BEAM SPAN

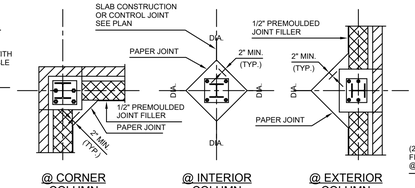
TYPICAL REINF. DETAIL FOR OPENINGS IN STEEL BEAMS



BEAM PARALLEL TO WALL | BEAM PERPENDICULAR TO WALL | BEAM PARALLEL TO WALL

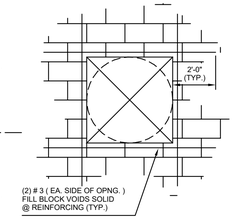
NOTE:
ALL STEEL (EXCEPT 10 GA. BENT PL.) SUPPLIED BY STEEL CONTRACTOR AND INSTALLED BY GENERAL CONTRACTOR. 10 GA. BENT PL. SUPPLIED AND INSTALLED BY GENERAL CONTRACTOR.

TYPICAL MASONRY WALL UP TO STEEL DECK DETAILS

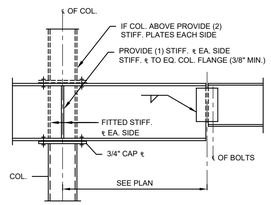


@ CORNER COLUMN | @ INTERIOR COLUMN | @ EXTERIOR COLUMN

TYPICAL ISOLATION JOINT DETAILS



TYPICAL REINFORCING @ OPENINGS IN MASONRY WALLS



CANTILEVER BEAM CONNECTION

ROOF LOADING SCHEDULE

DEAD LOAD:

METAL ROOF DECK	3.0	P.S.F.
ROOFING	2.0	P.S.F.
INSULATION	6.0	P.S.F.
STRUCTURAL STEEL	5.0	P.S.F.
MECH., ELEC.	2.0	P.S.F.
CEILING	2.0	P.S.F.
	20.0	P.S.F.

LIVE LOAD:

DESIGN LOAD	30.0	P.S.F.
GROUND SNOW LOAD (Pg)	25.0	P.S.F.
Ce = 1.0, I = 1.20 Ct = 1.0 Pf = 0.7 x Ce x I x Ct x Pg =	21.0	P.S.F.

LATERAL LOADING SCHEDULE

WIND LOAD: IBC 2021 - ASCE 7-16

BASIC WIND SPEED = 120 M.P.H.
RISK CATEGORY = IV , EXPOSURE B
INTERNAL PRESSURE COEFF. GCpi = ; 0.18
COMPONENT & CLADDING
ROOF - 10 SF = - 45.5 P.S.F.
WALLS - 10 SF = + 26.1, - 34.9 P.S.F.

SEISMIC LOADING: IBC 2021 - ASCE 7-16

SEISMIC RISK CATEGORY - IV
IMPORTANCE FACTOR (Ie) = 1.50
Ss = 0.176 S1 = 0.047
SITE CLASS - D
Sds = 0.188 Sd1 = 0.075
SEISMIC DESIGN CATEGORY - C
BUILDING FRAME SYSTEM: ORDINARY REINFORCED MASONRY SHEAR WALLS
BASE SHEAR = 84.63 KIPS
SEISMIC RESPONSE COEFFICENT (Cs) = 0.141
RESPONSE MODIFICATION FACTOR (R) = 2
EQ. LAT. FORCE PROC.

GENERAL NOTES

FOUNDATION

1. All footings shall bear on soil having a minimum safe bearing capacity of 1.5 tons per square foot. Contractor is responsible to hire geotechnical engineer to confirm in field prior to placing footings.
2. Elevations given correspond to the computed bottom of footings and are minimum depths which are not to be construed as limiting in any way the depth required to reach good bearing.
3. No footings shall be placed in water or on frozen ground.
4. No fill or backfill shall be placed over or against work at such time or in such a manner which would endanger the stability or otherwise damage such work.

UNDERPINNING NOTES

1. Existing wall to be underpinned the entire length as shown on the drawings in 3'-0 maximum sections. Decrease length of sections if pieces of existing foundation wall become loose.
2. Existing columns and piers adjacent to existing walls to be underpinned prior to underpinning any walls.
3. The procedure for underpinning the existing columns and piers are as follows. Divide column or pier footing into three equal sections across the width of the footing. Underpin the existing column or pier footings in the following sequence:
a. the outer right section,
b. the outer left section,
c. the center section.
Place concrete beneath each section of footing as specified in 5.
4. Underpin walls in alternating sequence, skipping two 3 foot sections between each underpinned section.
5. Place underpinning concrete in each pit to an elevation of 3 inches below bottom of existing foundations. Allow concrete to set for 24 hours and then "dry pack" the remaining 3 inches. Dry pack mix to be 1 part cement, 2 1/2 parts sand, and 1/10 parts water.
6. The contractor is responsible to shore the existing structure and soil excavation during construction.

CAST-IN-PLACE CONCRETE

1. All concrete work shall conform to the latest edition of the ACI Building Code.
2. All concrete, except slabs on grade, shall attain 3000 PSI compressive strength at 28 days. All concrete for slabs on grade shall attain 3500 PSI compressive strength at 28 days.

REINFORCING

1. All reinforcing bar details shall conform to the latest ACI code and detailing manual.
2. All bars shall be ASTM A-615, Grade 60.
3. Welded wire fabric shall be ASTM A-185.
4. Clearance of main reinforcing from adjacent surfaces unless shown otherwise shall be:
A. Unformed surfaces in contact with ground or exposed to the weather: 3"
B. Bottom surfaces of slabs on grade: 3"
C. Formed surfaces in contact with ground or exposed to weather:
1. #5 bars or smaller: 1-1/2"
2. Bars larger than #5: 2"
D. Exterior wall surfaces: 2"
E. In all cases not less than the diameter of the bar.
5. All reinforcement shall be inspected and approved before concrete is poured.
6. Tolerances for placing reinforcing shall be:
A. +or- 1/4 inch for members with an effective depth of 24 inches or less.
B. +or- 1/2 inch for members with an effective depth of more than 24 inches.
7. Where continuous bars are called for, they shall be run continuously around corners and lapped at necessary splices or hooked at discontinuous ends. Laps shall be 40 bar diameters. Bar laps may be offset to avoid control or construction joints.
8. Electrical Contractor to provide grounding electrode system as required by NEC section 250-52(a). Coordinate location and schedule with General Contractor.

STRUCTURAL STEEL

1. All structural steel details shall be designed in accordance with the latest issue of the American Institute of Steel Construction (AISC), "Specification for Structural Steel Buildings - Allowable Stress Design and Plastic Design".
2. Connections shall be designed to develop the full strength of the member over the required span, 1 1/2 times for composite members.
3. Provide double angle connections at all beam to wide flange columns and beam to beam connections whenever possible. The steel fabricator must notify the Structural Engineer if there are to be any changes. See Typical Thru-Pi Detail for beam to tube column connections.
4. Field connections shall be made by high strength bolts 3/4" minimum in diameter or welded as shown on drawings.
5. All pipe shapes shall be ASTM A53, Grade B, Fy= 35 KSI.
6. All tubes shapes shall be ASTM A500, Grade B, Fy=46 KSI.
7. All structural wide flange shapes to be ASTM A-992/A572, Grade 50.
8. All steel plates, angles, channels are to be ASTM A-36 unless indicated otherwise.

STEEL ROOF DECK

1. Deck shall be 20 gage galvanized 1.5" wide rib variety (Type B) continuous over at least 3 spans.
2. Deck to be installed as per manufacturer's recommendations.

STEEL FLOOR DECK

1. Deck shall be 20 gage galvanized equal to 2" Lok-2 Floor by Roof Deck, Inc. or P-3623 by Canam Steel with respect to load carrying capacity, deflection, slab weight and U.L. label. Submit data to Structural Engineer.
2. Deck at in fill in existing building shall be 9/16" -24 gage UFS deck by Roof Deck, Inc. with respect to load carrying capacity, deflection, slab weight and U.L. label. Submit data to structural Engineer.
3. Deck installation to be in strict accordance with manufacturer's recommendations.
4. Installation to be 3 span continuous.
5. All beams to have puddle welds at 12" O.C. (max).
6. Floor construction is unshored composite design. The steel will deflect under the weight of the concrete placed. The general contractor is to pour sufficient concrete, over and above the specified depth, to provide a level floor.

MASONRY REQUIREMENTS

1. All block work shall be in accordance with IBC2009 w/ NJ Modification and other applicable codes.
2. All block shall be lightweight aggregate and conform to ASTM C 90.
3. Mortar shall be ASTM C 270, Type M for below grade and Type M or S for above grade work.
4. Where block fill is called for on drawings, use Type M mortar or concrete with a compressive strength of 2500 PSI in accordance with ASTM C 476, and installed in accordance with ACI-531 for high or low lift procedures.
5. All masonry walls are to have a #4 vertical reinforcing bars at ends, at intersections at corners and at a maximum spacing of 10 feet on centers. These reinforcing bars are to be full height and grouted solid. These bars may be spliced provided that a 40 bar diameter lap is maintained.
6. All openings are to have two #3 vertical reinforcing bars within 16 inches of each side and a bond beam top and bottom with two #3 bars (unless noted otherwise on plan). All reinforcing is to extend past the opening a minimum of 24 inches.
7. All masonry walls are to have a bond beam with two #4 bars within 16 inches of the top of the wall.
8. All running bond masonry walls are to have horizontal reinforcing at every other course. Where masonry is laid in other than running bond, horizontal joint reinforcement is to be provided at every horizontal joint. The horizontal wall reinforcing shall be No. 9 gage "Dur-o-wall" or equivalent. Provide fabricated corner sections at all corners.
9. All wall bearing beams are to have the masonry wall grouted solid a minimum of 8 inches each side of bearing location and two #4 vertical reinforcing bars full height at each grouted cell (maximum of four bars).
10. Coordinate masonry with all trades requiring items to be built-in.

MISCELLANEOUS

1. Contractor shall verify all dimensions, sections and elevations on the job.
2. Consult the Architectural, Mechanical and Electrical drawings for verification of location and dimensions of chases, inserts, openings, sleeves, washes, drips, reveals, depressions, equipment pads and other product requirements.
3. All walls shall be braced during construction until permanently restrained.
4. Reproductions of contract documents are not acceptable as shop drawings and will be rejected.
5. Contractor is solely responsible for all necessary shoring of the building during during construction and the installation of new structural elements.

NOTES:
UNLESS OTHERWISE SPECIFIED ON DRAWINGS PROVIDE & INSTALL LINTELS FOR ALL SQUARE HEAD MASONRY OPENINGS IN ALL MASONRY WALLS IN ACCORDANCE WITH THE FOLLOWING SCHEDULES & COMMENTS.

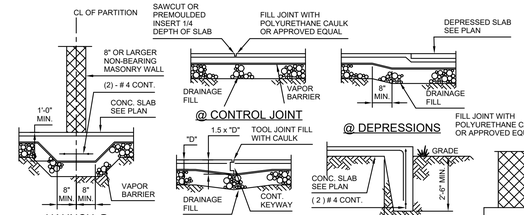
LOOSE LINTEL SCHEDULE
(FOR 4", 8", 12", & 16" WALLS)

MASONRY OPENING	LINTEL SIZE	REMARKS
UP TO 4'-0"	L - 3 1/2" x 3 1/2" x 1/4"	
4'-1" TO 6'-0"	L - 5" x 3 1/2" x 5/16"	
6'-1" TO 8'-0"	L - 6" x 3 1/2" x 5/16"	
OVER 8'-0"	W 8 x 18 + PL.	

LOOSE LINTEL SCHEDULE
(FOR 6" WALLS)

MASONRY OPENING	LINTEL SIZE	REMARKS
2'-0" TO 6'-0"	WT 7 x 11	
6'-1" TO 8'-0"	WT 8 x 13	

NOTES:
1. PROVIDE ONE (1) ANGLE FOR EACH 4" OF MASONRY WIDTH.
2. BEAR LINTELS 6" MINIMUM EACH SIDE OF OPENING.
3. ALL LINTELS IN EXTERIOR WALLS TO BE GALVANIZED.
4. WHERE OPENINGS LOCATED NEXT TO COLUMNS OR BEAMS, ATTACH TO STRUCTURAL STEEL, CONNECTION NOT TO PROTRUDE INTO OPENING.
5. CONSULT ARCHITECTURAL, MECHANICAL & ELECTRICAL DRAWINGS FOR OPENING SIZE & LOCATION.



HAUNCH @ PARTITIONS

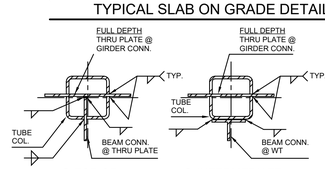
@ CONTROL JOINT | @ CONSTRUCTION JOINT

@ DEPRESSIONS | @ EXTERIOR PLATFORM

@ FOUNDATION WALL & SLAB

RADON

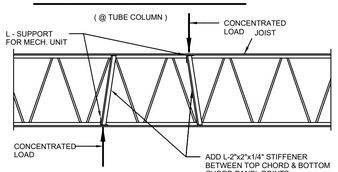
TYPICAL SLAB ON GRADE DETAILS



@ TOP OF COLUMN | @ INTERMEDIATE FRAME

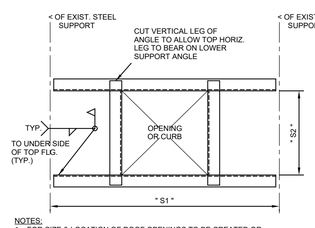
NOTE:
WHERE A SINGLE ROW OF ¾"DIA. BOLTS DOES NOT MEET THE CONNECTION REQUIREMENTS STATED IN THE GENERAL NOTES PROVIDE A FULL DEPTH SINGLE ROW OF 1"DIA. BOLTS BEFORE USING A DOUBLE ROW OF BOLTS

TYPICAL THRU PLATE DETAIL



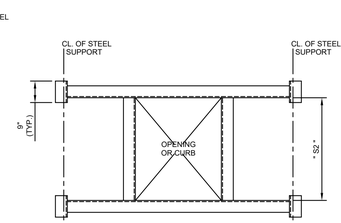
NOTE:
SUPPORTING LOAD AT CONDENSERS, MECHANICAL UNITS AND ALL CONCENTRATED LOADS GREATER THAN 300 lbs. OMIT IF LOAD IS WITHIN 3" OF A PANEL POINT.

TYPICAL DETAIL OF REINF'D. JOIST



NOTES:
1. FOR SIZE & LOCATION OF ROOF OPENINGS TO BE CREATED OR INFILLED SEE ARCHITECTURAL & MECHANICAL DRAWINGS.
2. STEEL CONTRACTOR SHALL VERIFY ALL OPENINGS AND EXACT LOCATIONS WITH THE TRADE CONTRACTOR REQUIRING SAID OPENINGS PRIOR TO FABRICATION & ERECTION OF STEEL FRAMES.
3. PROVIDE STEEL FRAMES AS SHOWN AROUND ALL REQUIRED OPENINGS LARGER THAN 8" AT ROOF.
4. PROVIDE STEEL ANGLES ON ALL SIDES OF OPENINGS UNLESS BEAM IS SHOWN ON PLAN. ANGLE SIZES TO BE:
FOR "S1" OR "S2" ≤ 6'-6" L-4"x4"x5/16"
FOR "S1" OR "S2" > 6'-6" L-6"x6"x3/8"
5. PROVIDE STEEL ANGLES ON ALL SIDES OF MECH'L CURBS UNLESS BEAM IS SHOWN ON PLAN. ANGLE SIZES TO BE L-6"x6"x3/8".
6. FOR SIZE & LOCATION OF MECH'L UNITS, SEE ARCH'L & MECH'L DWGS.
7. PROVIDE L-FRAMES PER TYPICAL DETAIL FOR INFILL OF EXISTING ROOF OPENINGS. PROVIDE 1 1/2"-20 GA. TYPE "B" DECK (WIDE RIB) METAL ROOF DECK (GALV.).

TYPICAL ROOF OPNG., CURB SUPPORT AND ROOF INFILL DETAIL @ EXIST'G. ROOF



NOTES:
1. FOR SIZE & LOCATION OF OPENINGS SEE ARCHITECTURAL & MECHANICAL DRAWINGS.
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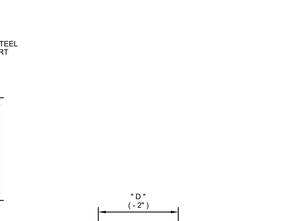
5. PROVIDE STEEL ANGLES ON ALL SIDES OF MECH'L CURBS UNLESS BEAM IS SHOWN ON PLAN. ANGLE SIZES TO BE L-6"x6"x3/8".
6. FOR SIZE & LOCATION OF MECH'L UNITS, SEE ARCH'L & MECH'L DWGS.

TYPICAL DETAIL @ ROOF OPENING & UNDER MECH'L CURB



NOTE:
LOCATE OPENINGS IN MIDDLE 1/3 OF BEAM SPAN

TYPICAL REINF. DETAIL FOR OPENINGS IN STEEL BEAMS

TYPICAL COLUMN UP FROM BEAM

CODE REVIEW:

CERTIFICATE:

spiezle

SPIEZLE ARCHITECTURAL GROUP INC.
121 MARKET STREET
CAMDEN, NJ 08102
PHONE: (866) 974 7666

SIGNATURE:

THOMAS S. PERRINO	21A01505400
SCOTT E. DOWNIE	21A01674400
STEVEN LEONE	21A01170100
STEVEN G. SIEGEL	21A01564200
ANGELO ALBERTO	21A01046700
JOHN F. WRIGHT	21A01784200
SPIEZLE ARCHITECTURAL GROUP, INC.	21AZ00083000

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MARK W. GAFFNEY, NJPE #24GE04284100 DATE

BID SET - 06/25/2024

PROJECT:

NEW HADDONFIELD POLICE STATION

1 WALNUT STREET, HADDONFIELD, NJ 08033

FOR

CAMDEN COUNTY IMPROVEMENT AUTHORITY

2120 VOORHEES TOWN CENTER, VORHEES TOWNSHIP, NJ 08043

FOR CODE REVIEW: 02/23/24

REVISIONS:

#	REVISION NAME	DATE

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DRAWING TITLE:

TYPICAL DETAILS, GENERAL NOTES & SCHEDULES

COMMISSION NUMBER:

23M014

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