FOUNDATION NOTES:

1. FOUNDATION SOIL BEARING PRESSURE 2500 PSF. (ASSUMED, TO BE FIELD VERIFIED)

ELEVATIONS INDICATED ON DRAWINGS.

PLACEMENT OF CONCRETE FOOTINGS.

- FOUNDATIONS SHALL BE PLACED ON VIRGIN SOIL OR STRUCTURAL FILL AT
- 3. ALL STRUCTURAL FILL SHALL BE COMPACTED TO A DENSITY OF 95% OF THE MAXIMUM MODIFIED PROCTOR DENSITY AS DEFINED BY ASTM D-1557.
- 4. THE SOILS ENGINEER SHALL APPROVE ALL BEARING STRATA PRIOR TO
- NO GEOTECHNICAL REPORT AVAILABLE AT TIME OF DESIGN. 2500 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. CONTRACTOR IS TO VERIFY SOIL BEARING PRESSURE AND INFORM ENGINEER IF ACTUAL CONDITIONS DO NOT MEET OR EXCEED ASSUMED VALUE. CONTRACTOR TO COORDINATE GEOTECHNICAL REQUIREMENTS WITH PROJECT DRAWINGS INCLUDING ANY AND ALL REQUIRED SUBSURFACE SOIL PREPARATION, MODIFICATIONS, IMPROVEMENTS OR REPLACEMENTS.

CONCRETE NOTES:

- 1. MATERIALS: 4000 PSI @ 28 DAYS CONCRETE SLAB ON GRADE ALL OTHER CONCRETE 3000 PSI @ 28 DAYS REINFORCING STEEL ASTM A615, GRADE 60 WELDED WIRE FABRIC ASTM A185
- REINFORCED CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH 2. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 318, LATEST EDITION.
- HORIZONTAL REINFORCING BARS IN WALLS AND FOOTINGS SHALL BE CONTINUOUS AROUND CORNERS.
- 4. REINFORCING BARS SHALL BE LAPPED WITH A MINIMUM OF 36 BAR DIAMETERS AT SPLICES AND WELDED WIRE FABRIC SHALL BE LAPPED A MINIMUM OF 6 INCHES UNLESS NOTED OTHERWISE ON DRAWINGS.
- 5. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR **REINFORCEMENT:** CONCRETE CAST AGAINST EARTH & PERMANENTLY EXPOSED 3" CONCRETE EXPOSED TO EARTH OR WEATHER #5 & SMALLER #6 & LARGER CONCRETE NOT EXPOSED TO WEATHER OR GROUND SLABS AND JOISTS BEAMS AND WALLS COLUMNS AND PILASTERS SLABS ON GRADE 2" MAX. (TOP)
- 6. MINIMUM EMBEDMENT LENGTH SHALL BE 24 BAR DIAMETERS UNLESS OTHERWISE NOTED.
- 7. ALL CONCRETE EXPOSED TO THE WEATHER SHALL BE AIR-ENTRAINED 4%-6%.
- 8. ALL REINFORCING SHALL BE SUPPORTED ON REINF. CHAIRS OR CONCRETE MASONRY UNITS.
- 9. ALL REINFORCING SHALL BE TIED AND SET IN PLACE PRIOR TO PLACING CONC.
- 10. CONCRETE SHALL NOT BE PLACED ON WET OR FROZEN SUBSTRATE.
- 11. CONCRETE SHALL BE PLACED ONLY AFTER INSPECTION AND APPROVAL OF SUBSTRATE FORM WORK, REINFORCING, AND EMBEDMENTS BY TOWNSHIP ENGINEER OR FIELD REPRESENTATIVE.
- 12. CONCRETE SHALL BE TESTED AT A MINIMUM FOR COMPRESSIVE STRENGTH, SLUMP AND AIR ENTRAINMENT FOR EACH DAY'S PLACEMENT AND FOR EACH FIFTY (50) CUBIC YARDS PLACED DURING A DAY
- 13. CONCRETE WALLS SHALL BE IN PLACE FOR A MINIMUM OF 14 DAYS AND AFTER FLOOR FRAMING IS INSTALLED PRIOR TO BACKFILLING. AT CONTRACTORS OPTION, BACKFILL MAY BE PLACED AGAINST WALLS THAT HAVE BEEN TEMPORARILY SHORED; HOWEVER, CONTRACTOR SHALL BE RESPONSIBLE FOR CORRECTING ANY MOVEMENT OF WALLS DUE TO BACKFILLING.
- 14. ALL REINF. DETAILS SHALL CONFORM TO THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" (ACI 315) UNLESS DETAILED ON THE STRUCTURAL DRAWINGS

MASONRY NOTES:

1.	MATERIALS:	
	CONCRETE MASONRY UNITS	ASTM C90 GRADE N
		(MIN. COMP. STRENGTH = 1900 PSI)
	MORTAR	ASTM C270 TYPE M OR S
		(MIN. COMP. STRENGTH = 2500 PSI)
	GROUT	ASTM C476
		(MIN. COMP. STRENGTH = 3000 PSI)

- 2. MASONRY CONSTRUCTION SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES" AND "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY" AS PUBLISHED BY THE NATIONAL CONCRETE MASONRY ASSOCIATION.
- 3. ALL MASONRY BEARING BENEATH STEEL COLUMN, BEAM AND LINTEL SUPPORTS SHALL HAVE THREE COURSES OF CMU FILLED WITH CONCRETE GROUT OR SHALL BE 100% SOLID CMU, UNLESS NOTED OTHERWISE.
- 4. ALL HORIZONTAL WALL REINFORCING SHALL BE TRUSS TYPE, GALVANIZED REINFORCING SPACED AT 16" O.C. VERTICALLY. AT ALL CORNERS AND INTERSECTIONS HORIZONTAL WALL REINFORCING SHALL BE FULLY LAPPED WITH TRUSS TYPE, GALVANIZED CORNERS AND TEES.
- 5. PROVIDE MASONRY ANCHORS AT 1'-4" O.C. MAXIMUM, SET ON COURSING AND WELD TO ALL BEAMS AND COLUMNS ABUTTING OR EMBEDDED IN MASONRY.
- 6. MASONRY WALLS SHALL BE IN PLACE FOR A MINIMUM OF 14 DAYS AND AFTER FLOOR FRAMING IS INSTALLED. PRIOR TO BACKFILLING. AT CONTRACTORS OPTION, BACKFILL MAY BE PLACED AGAINST WALLS THAT HAVE BEEN TEMPORARILY SHORED; HOWEVER, CONTRACTOR SHALL BE RESPONSIBLE FOR CORRECTING ANY MOVEMENT OF WALLS DUE TO BACKFILLING.
- 7. PROVIDE WALL CONTROL JOINTS @ 30'-0" MAX O.C., WITHIN 10'-0" OF ALL CORNERS, AND ON ONE SIDE OF ALL LARGE OPENINGS. COORD. LOCATION WITH ARCH. ELEVATIONS. FILL WITH BACKER ROD AND SEALENT PER ARCH'L SPECS.

STRUCTURAL STEEL NOTES:

- 1. MATERIALS:
 - BEAMS LINTELS AND ANGLES TUBE/HSS STEEL PIPE COLUMN ANCHOR BOLTS HIGH-STRENGTH BOLTS
 - WELDING ELECTRODES
- BEAM-TO-BEAM AND BEAM-TO-COLUMN CONNECTIONS SHALL BE AISC STANDARD FULL DEPTH DOUBLE ANGLE CONNECTIONS. WHERE REACTIONS EXCEED MINIMUM CONDITIONS, THE APPROPRIATE CONNECTION SHALL BE DETERMINED BY FABRICATOR (CONTRACTOR).
- 3. EQUAL STRENGTH. ANCHOR BOLTS SHALL BE UNFINISHED BOLTS.
- 4. ALL COLUMNS TO BE PROVIDED WITH 3/4" THICK CAP PLATES, AS REQ'D
- AISC SPECIFICATIONS.
- ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY USING E70XX ELECTRODES.
- SHOP AND ERECTION DRAWINGS MUST SHOW ALL SHOP AND FIELD WELDS.
- 8. PROVIDE MASONRY ANCHORS AT 1'-4" o.c. MAXIMUM,SET ON COURSING AND WELD TO ALL BEAMS AND COLUMNS ABUTTING OR EMBEDDED IN MASONRY
- 9. ALL STEEL TO RECEIVE ONE COAT OF SHOP PRIMER, FINAL FINISH TO BE COORDINATED WITH OWNER.
- 10. FOR ALL ARCH'L EXPOSED STEEL (HANGERS, PLATES, ETC.) REMOVE ALL BURRS, ETC. AND GROUND ALL WELDS SMOOTH.

WOOD FRAMING NOTES:

1. STANDARDS: LATEST EDITION.

2. MATERIALS: ALL EXPOSED WOOD AND ALL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED TO A MINIMUM .4 PCF (SEE PLANS FOR SIZES AND SPACINGS) STUDS, PLATES, BRIDGING AND BRACING: HEM-FIR NO. 2.

- VERIFIED BY STAMP.
- PLYWOOD: CONFORMANCE TO DFPA STANDARD.
- CONNECTIONS SHALL BE MINIMUM ASTM-A325.

TRUSS FABRICATOR SHALL SUBMIT STRESS DIAGRAMS AND SHOP DRAWINGS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION. PROVIDE BRIDGING IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS MEETING OR EXCEEDING TPI 78 SPECIFICATION.

- a. MATERIALS SHALL COMPLY WITH NES REPORT NO. NER-481 OR CCMC REPORT NO.
 - - BENDING STRESS.
 - HORIZONTAL SHEAR STRESS, FC=285 PSI; MODULUS OF ELASTICITY = 2X106 PSI

 - LENGTH MEMBERS.
 - WELL AS CONTINUOUSLY ALONG THE COMPRESSION FACE. e. OF ASTM D-2559.
 - ENGINEERING SHALL NOT BE PERMITTED.
- 5. WOOD I-JOISTS:

 - APPROPRIATE CCMC NUMBER. SUBMIT SHOP DRAWINGS SHOWING LAYOUT AND DETAIL NECESSARY FOR DETERMINING b.
 - FIT AND PLACEMENT IN THE BUILDING.
 - ERECTION AND INSTALLATION OF TJI I-JOIST.
- 6. ALL WORK SHALL CONFORM TO CODE REQUIREMENTS AITC 102, 105 & 108 STANDARD PRACTICE, AND APA CONSTRUCTION GUIDE.
- 7. ALL EXTERIOR BEARING WALLS TO BE CONSTRUCTED OF 2X6'S @ 16" O.C., WITH SOLID BLOCKING @ 4'-0" O.C.
- 8. COORDINATE LOCATION OF ALL BEARING WALLS, POSTS, COLUMNS, HEADERS, SHEAR WALLS, ETC. W/ ARCH. DWGS.
- 9. PROVIDE STRUCTURAL PLYWOOD SHEATHING OR APPROVED EQUAL AT ALL SIDES OF CORNERS FOR WIND BRACING. CONNECTIONS OF PLYWOOD SHALL COMPLY WITH APA NAILING REQUIREMENTS FOR PLYWOOD SHEAR WALLS.

- 4. PREFABRICATED PARALLEL STRAND WOOD MEMBERS: 11161-R. VALUES:
 - 3. PREFABRICATED TRUSSES:

ASTM ASTM	A992, A36	GRADE 50
ASTM	A-500,	GRADE B
ASTM	A53,	GRADE B
ASTM	A307	
ASTM	A325	
ASTM	A233,	CLASS E70

ALL MAJOR CONNECTIONS SHALL BE HIGH-STRENGTH FRICTION BOLTS OR WELDS OF

STEEL WORK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST

7. SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION AND ORDERING.

a. "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION. b. IBC BASIC BUILDING CODE. LATEST EDITION, "RECOMMENDED NAILING SCHEDULE". c. "TIMBER CONSTRUCTION MANUAL", AMERICAN INSTITUTE OF TIMBER CONSTRUCTION,

MOISTURE CONTENT OF ALL STRUCTURAL LUMBER SHALL BE LESS THAN 19% AS

HARDWARE: ALL EXPOSED HARDWARE SHALL BE HOT DIPPED GALVANIZED, NOT LESS THAN 18 GA., MFG'D BY THE SIMPSON COMPANY, ALL BOLTED

b. THE MATERIAL PROPERTIES SHALL MEET THE FOLLOWING MINIMUM STRUCTURAL

FB=2800 PSI;

c. PARALLAM PSL SHALL BE MANUFACTURED FROM STRANDS OF WOOD FIBER IN A CONTINUOUS PROCESS WITH ALL STRANDS ORIENTED TO THE LENGTH OF THE MEMBER AND THEN FED INTO A PRESS IN THE DESIRED ALY-UP PATTERN. ALL MEMBERS ARE TO BE FREE OF FINGER JOINTS OR SCARFS OR MECHANICAL CONNECTIONS IN FULL

d. ALL MEMBERS SHALL HAVE LATERAL SUPPORT SUPPLIED AT ALL BEARING POINTS AS ADHESIVES SHALL BE OF THE WATERPROOF TYPE CONFORMING TO THE REQUIREMENTS

f. HOLES, CUTS OR NOTCHES NOT PREVIOUSLY APPROVED BY TRUS JOIST MACMILLAN

a. I-JOISTS SHALL BE DESIGNED AND MANUFACTURED TO THE STANDARDS SET FORTH IN THE NATIONAL EVALUATION SERVICE, INC. NES REPORT NO. NER-200 OR THE

c. REFER TO ALL PERTINENT INFORMATION PROVIDED BY THE MANUFACTURER FOR

PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSS NOTES

- 1. CONTRACTOR SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. DRAWINGS SHALL BE COMPLETE IN ALL DETAILS, INCLUDING ALL BRACING LOCATIONS AND BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE PROJECT AND SHALL BE SOLELY RESPONSIBLE FOR SAME.
- TRUSS SUBCONTRACTOR SHALL FURNISH GALVANIZED METAL PLATES OF GAUGE AND SIZE TO FULLY DEVELOP CONNECTIONS FOR STRESS, PLATES SHALL BE ON BOTH SIDES OF CONNECTIONS.
- 3. FLOOR/ROOF CONSTRUCTION SHALL BE TEMPORARILY BRACED PER TRUSS MANUF. RECOMMENDATIONS DURING ERECTION UNTIL DECK IS RIGIDLY IN PLACE. DO NOT APPLY ANY TEMPORARY CONCENTRATED LOADS (BUNDLED PLYWOOD, ETC.) TO UN-BRACED OR UN-SHEATHED ROOF AREAS.
- 4. FURNISH ALL BRIDGING TIES, ANCHORS, ETC. TO MAKE A COMPLETE ROOF SYSTEM. FOR GABLE END TRUSS CONNECTION TO EXTERIOR WALL, USE SIMPSON A35F CONNECTORS @ 24" O.C. (OR APPROVED EQ.). FOR "PIGGY-BACK" TRUSSES, USE SIMPSON LTP4 CONNECTORS (OR APPROVED EQ.) TO CONNECT TO MAIN TRUSS.
- 5. FURNISH CONNECTOR PLATES AS SPECIFIED TO FULLY DEVELOP ALL TRUSS CONNECTIONS. SUBMIT STRESS DIAGRAM OF TRUSSES.
- 6. PERMANENT BRACING/BRIDGING SHALL BE 2x6 HORIZONTAL AT SPACING REQ'D BY TRUSS MANUFACTURER (12'-0" O.C. MAX.). BRACING TO BE NAILED TO BOTTOM CHORDS AND WEBS OF TRUSSES WITH MIN. (2)-10D NAILS AND BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND HIB-91. CONTRACTOR MUST FOLLOW AND BE RESPONSIBLE FOR ALL MANUFACTURER'S RECOMMENDATIONS FOR BRACING AND ERECTION OF THE WOOD TRUSSES.
- 7. ERECTION AND FABRICATION OF ALL LUMBER SHALL BE IN ACCORDANCE WITH A.I.T.C. SPECIFICATIONS AND RECOMMENDATIONS (LATEST EDITION) AND HIB-91.
- 8. PROVIDE DOUBLE WOOD TRUSSES WITH REINFORCED CHORDS AT ALL MECHANICAL UNITS THAT ARE TO BE HUNG OR REST ON ANY WOOD TRUSSES. SHOW ALL MECHANICAL UNITS ON DESIGN DRAWINGS. PROVIDE ADDITIONAL "X" BRACING AT ALL MECHANICAL UNIT LOCATIONS. CONTRACTOR MUST NOTIFY THE TRUSS MANUFACTURER OF ANY ADDITIONAL MECHANICAL UNITS TO BE PLACED ON THE WOOD TRUSSES NOT SHOWN ON DESIGN DRAWINGS PRIOR TO LOADING THE TRUSSES AND OBTAIN APPROVAL FROM THE TRUSS MANUFACTURER'S ENGINEER.
- 9. ALL WOOD TRUSSES TO BE SPACED @ 24" O.C. MAX.; FLOOR TRUSSES @ 16" O.C. MAX., U.N.O.
- 10. TRUSS DESIGN LOADS:

ROOF	<u>FLOOR</u>
TC LL = 30 PSF	TC LL = 50 PSF
TC DL = 10 PSF	TC DL = 20 PSI
BC DL = 10 PSF	BC DL = 5 PSF

11. COORDINATE ALL ROOF TRUSS LAYOUTS, PROFILES, SLOPES OVERHANGS, AND OVERFRAMING WITH ARCH'L

TYPICAL NAILING SCHEDULE <u>CONNECTION</u>

FT5.5

BP4

5'-6"x5'-6"x1'-4"

4'-0"x4'-0" BULKPOUR

1)	JOIST TO SILL OR GIRDER, TOENAIL	(3–8[
2)	BRIDGING TO JOIST, TOENAIL EACH END	(2-8[
3)	1"X6" (25MM X 152MM) SUBFLOOR OR LESS TO JOIST, FACE NAIL	(2-8[
4)	WIDER THAN 1" X 6"(25MMX152MM)SUBFLOOR TO JOIST, FACE NAIL	(3-8[
5)	2"(52MM) SUBFLOOR TO GIRDER, BLIND AND FACE NAIL	(2-16
6)	SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL	(16D @ 16"O.C. (3)-16D PER 16
7)	TOP PLATE TO STUD. END NAIL	(2-16
8)	STUD TO SOLE PLATE	(4-8D, TOENAIL OR 2-16D, END NAIL
9)	DOUBLE STUDS, FACE NAIL	(16D @ 24" O.C
10)	DOUBLED TOP PLATES, TYPICAL FACE NAIL	(16D @ 16" O.C
	DOUBLE TOP PLATES, LAP SPLICE	(8–16
11)	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL	(3-8[
12)	RIM JOIST TO TOP PLATE, TOENAIL	(8D @ 6" O.C
13)	TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	(2-16[
14)	CONTINUOUS HEADER, TWO PIECES	(16D @ 16" O.C. ALONG E. EDG
15)	CEILING JOISTS TO PLATE, TOENAIL	(3–8[
16)	CONTINUOUS HEADER TO STUD, TOENAIL	(4-8[
17)	CEILING JOISTS, LAP OVER PARTITIONS, FACE NAIL	(3–16[
18)	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	(3–16[
19)	RAFTER TO PLATE, TOENAIL	(3-8[
20)	1"(25MM) BRACE TO EACH STUD AND PLATE, FACE NAIL	(2-8[
21)	1"X 8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	(2-8[
22)	WIDER THAN 1X8" SHEATHING TO EACH BEARING, FACE NAIL	(3-8[
23)	BUILT-UP CORNER STUDS	(16D @ 24" O.C
24)	BUILT-UP GIRDER AND BEAMS	20D @ 32" O.C. AT TOP AN
		BOTTOM AND STAGGARD (2)-20
		AT ENDS AND AT EACH SPLIC
25)	2"PLANKS	(2—16D AT EACH SPLICE

	PIER SCHEDULE						
		Mk	SIZE	VERT. REINF.	HOR. TIES		REMARKS
	Γ	P1	1'-6" × 1'-6"	(6)-#6's	#4'S @ 8" 0.0	С.	
FOOTING SCHEDULE fc=3000 PSI / Fb=2,000 PSF							
ML	NIK SIZE REINFORCING		DEMADIZS				
IVIK			A x B x t	SHORT WAY	LONG WAY		ILIMANNS
FT4		4'-	0"x4'-0"x1'-0"	(5)-#5's	(5)- # 5's		
FT4.5		4'—	6"x4'-6"x1'-4"	(6)-#6's	(6)-#6's		

(7)-#6's (5)-#5's

HEADER & LINTEL SCHEDULE

Mk	MATERIAL	REMARKS	POST
H1	(3)-2x8's W/½" PLYWOODS		(2) – 2x6 SUPPORT AT EACH END MIN.
H2	(3)-2x10's W/ ½" PLYWOODS		(3) – 2x6 SUPPORT AT EACH END MIN.
Н3	PRE-CAST CONC. OR CMU HEADER		PROVIDE 8" MIN. BRG., EACH END

(7)-#6's

(5)-#5's

SEE DET. 10/S3.2

<u>DESIGN DATA (IB</u> (new jersey edit	<u>C 2021)</u> ION)	
<u>1ST & 2ND FLOOR</u> live load (office) dead load	<u>50</u> LBS/FT ² 20 LBS/FT ²	& PARTNERS
		ARCHITECTURE · PLANNING · INTERIOR DESIG
LIVE LOAD (SNOW) DEAD LOAD	<u>30</u> LBS/FT ² 20 LBS/FT ²	1225 NORTH BROAD STREET SUITE 4 WEST DEPTFORD NJ 08096 t: 856 384 1225
Building category		e-mail: info@ruggieriandpartners.com website: www.ruggieriandpartners.com
ROOF SNOW LOAD DRIFTING SNOW IN Pg = 30 LBS./FT. ² Order of the state of	ADDITION TO HERE APPLICABLE)	SALVATORE DIGENOVA Engineer Registration no. 42619 Date:Signed
COLUMNS WALLS	<u>3000 LBS./FT</u> 2 <u>3000 LBS./FT</u> 2	CLIENT:
EARTHQUAKE DESIGN DATA SEISMIC USE GROUP I SEISMIC DESIGN CATEGORY B SDS = 0.174 SD1 = 0.087 SITE CLASS D BASIC SEISMIC-FORCE-RESISTING SYSTEM = LIGHT-FRAME WALLS SHEATHED W/ WOOD RATED FOR SHEAR RESISTANCE IE= FOULVALENT LATERAL FORCE PROCEDURE	= STRUCTURAL PANELS	Allied Painting

LEGEND:

В.О.	BOTTOM OF
B.O.F.	BOTTOM OF FOOTING
BS	BOTH SIDES
CJ	CONTROL JOINT
С <u></u>	CENTER LINE
E.F.	EACH FACE
FT	FOOTING
FND	FOUNDATION
MJ	MASONRY WALL CONTROL JOINT
MP	MASONRY PIER
0.C.	ON CENTER
0.F.	OUTSIDE FACE
P	PIER
Æ	PLATE
	DENOTES CMU
	DENOTES CONCRETE
/////	DENOTES BRICK

4 Larwin Road Cherry Hill, NJ 08034
PROJECT:

Proposed Warehouse Facility

2174 South Black Horse Pike Block 3901, Lot 29 Monroe Township, Gloucester County NJ 08094

REVISIONS:

A 11/18/24 PRELIM. PRICING / BID SET

SHEET TITLE:

General Notes and Specs

DRAWN BY:	SLR
CHECKED BY:	SD
SCALE:	AS NOTED
DATE:	9-03-2024
PROJECT NUMBER:	2024-058.00

DRAWING NUMBER:



DI GENOVA

Associates, inc. STRUCTURAL ENGINEERING + DESIGN 1212 Germantown Pike, Suite 5 Plymouth Meeting, PA 19462 610.270.9511Phone 610.270.9657 Fax

<u>NAILING</u>



DRAWN BY:	SLR
CHECKED BY:	SD
SCALE:	AS NOTED
DATE:	9-03-2024
PROJECT NUMBER:	2024-058.00





1225 NORTH BROAD STREETSUITE 4WEST DEPTFORDNJ08096t: 856.384.1225e-mail:info@ruggieriandpartners.comwebsite:www.ruggieriandpartners.com

SALVATORE DIGENOVA ENGINEER REGISTRATION NO. 42619 DATE: ______SIGNED

CLIENT:

Allied Painting

4 Larwin Road Cherry Hill, NJ 08034

PROJECT: Proposed Warehouse Facility

2174 South Black Horse Pike Block 3901, Lot 29 Monroe Township, Gloucester County NJ 08094

REVISIONS:

A 11/18/24 PRELIM. PRICING / BID SET

SHEET TITLE:

Second Floor Framing Plan

DRAWN BY:	SLR
CHECKED BY:	SD
SCALE:	AS NOTED
DATE:	9-03-2024
PROJECT NUMBER:	2024-058.00

DRAWING NUMBER:



1 SECOND FLOOR FRAMING PLAN SCALE: 1/4"=1'-0"

NOTES:

- 1. ALL EXTERIOR WALLS SHALL BE 2x6's ◎ 16" O.C. AND WITH SOLID BLOCKING ◎ 4'-0" O.C.
- 2. ALL STEEL BEAMS SUPPORTING FLOOR FRAMING TO BE "FLUSH" AND TO BE PROVIDED WITH BOLTED 2X TOP PL AND BOLTED FULL DEPTH WEB BLOCKING.
- 3. FLOOR CONSTRUCTION SHALL BE $\frac{3}{4}$ " T&G PLYWOOD
- 4. TRIPLE 2x WALL STUD FRAMING TO SUPPORT ALL PARALLAMS, HEADERS, ETC.
- 5. COORD. LOCATION OF ALL WALLS AND DIMENSIONS
- WITH ARCHITECTURAL DRAWINGS 6. FLOOR TRUSS BEARING ELEV.= COORD. W/ ARCH'L.
- 7. T.O. STEEL ELEV. = COORD. W/ARCH'L.
- 8. PROVIDE BOLTED 2X WEB BLOCKING, FULL DEPTH, ON "EXTERIOR SIDE."

LEGEND:



DI GENOVA ASSOCIATES, INC. STRUCTURAL ENGINEERING + DESIGN 1212 Germantown Pike, Suite 5 Phymouth Meeting, PA 19462 610.270.9657 Fax





1225 NORTH BROAD STREET		SI	UITE 4
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website:	www.ruggieriandpartners.com		

SALVATORE DIGENOVA ENGINEER REGISTRATION NO. 42619

DATE: _____ SIGNED

CLIENT:

Allied Painting

4 Larwin Road Cherry Hill, NJ 08034

PROJECT: Proposed Warehouse Facility

\ROOF FRAMING PLAN S-2.1 SCALE: 1/4"=1'-0"

NOTES:

- 1. ALL EXTERIOR WALLS SHALL BE 2x6's @ 16" O.C. AND WITH SOLID BLOCKING @ 4'-0" O.C., VERT.
- 2. ALL INTERIOR BRG. WALLS SHALL BE 2x6's @ 16" O.C. AND WITH SOLID BLOCKING @ 4'-0" O.C., VERT.
- 3. TRIPLE 2x WALL STUD FRAMING TO SUPPORT ALL PARALLAMS, HEADERS, ETC., U.N.O. ON PLAN
- 4. SHADED WALLS INDICATE THUS (
- 5. TRUSS BEARING ELEV. = COORD. W/ ARCH'L. T.O. STEEL ELEV. = COORD. W/ARCH'L.
- TRUSS ROOF FRAMING LAYOUT SHOWN ABOVE IS SCHEMATIC AND FOR INFO 6. ONLY. SEE ARCH'L DWG'S FOR ADDITIONAL INFORMATION ON TRUSS PROFILE, SLOPE DIMENSIONS, ETC. ACTUAL ROOF TRUSS LAYOUT AND LOCATIONS OF GIRDER DOUBLE TRUSS, STEP-DOWN TRUSSES, JACK TRUSSES, ETC. TO BE PROVIDED BY TRUSS MANUFACTURER.
- (7.) 2x6 PERMANENT LATERAL BOT. CHORD TRUSS BRACING @ 12'-0" O.C. MAX. (OR AS REQ'D BY TRUSS MANUFAC. ATTACH TO EACH TRUSS WITH MIN. (2)-10d NAILS.
- 8. 2x6 DIAGONAL TRUSS BRACING @ 20'-0" O.C. MAX. (CONNECT W/ (2)-10d NAILS AT ALL TRUSS MEMBERS
- 9. ALL STEEL BEAMS SUPPORTING ROOF FRAMING TO BE "DROPPED" AND PROVIDED WITH BOLTED 2x TOP PL.
- (10) PROVIDE BOLTED 2X WEB BLOCKING, FULL DEPTH, ON "EXTERIOR SIDE."

DI GENOVA

Associates, inc.

STRUCTURAL ENGINEERING + DESIGN
 1212 Germantown Pike, Suite 5
 610.270.9511Phone

 Plymouth Meeting, PA 19462
 610.270.9657 Fax

(11) PROVIDE 2x8 @ 24" O.C. INFILL FRAMING OVER ELEVATOR SHAFT.

LEGEND:

]>×<[-	DENOTES SHEAR WALL LOCATION (SEE 10/S3.0)
-	2x6 STUDS @ 16" O.C. EXTERIOR BEARING WALL
(Hx) –	DENOTES HEADER (SEE HEADER SCHEDULE ON S–0.0)
▶ -	DENOTES MOMENT CONNECTION (SEE 8/S3.2)

2174 South Black Horse Pike Block 3901, Lot 29 Monroe Township, Gloucester County NJ 08094

REVISIONS:

A 11/18/24 PRELIM. PRICING / BID SET

SHEET TITLE:

Roof Framing Plan

DRAWN BY:	SLR
CHECKED BY:	SD
SCALE:	AS NOTED
DATE:	9-03-2024
PROJECT NUMBER:	2024-058.00

DRAWING NUMBER:

S-2.1



DRAWN BY:	SLR
CHECKED BY:	SD
SCALE:	AS NOTED
DATE:	9-03-2024
PROJECT NUMBER:	2024-058.00









1225 NORTH BROAD STREET SUITE 4 WEST DEPTFORD NJ 08096 t: 856.384.1225 e-mail: info@ruggieriandpartners.com website: www.ruggieriandpartners.com

> SALVATORE DIGENOVA ENGINEER REGISTRATION NO. 42619 DATE: ______SIGNED

CLIENT:

Allied Painting

4 Larwin Road Cherry Hill, NJ 08034

PROJECT: Proposed Warehouse Facility

2174 South Black Horse Pike Block 3901, Lot 29 Monroe Township, Gloucester County NJ 08094

REVISIONS:

A 11/18/24 PRELIM. PRICING / BID SET

SHEET TITLE:

Sections and Details

DRAWN BY:	SLR
CHECKED BY:	SD
SCALE:	AS NOTED
DATE:	9-03-2024
PROJECT NUMBER:	2024-058.00

DRAWING NUMBER:



DI GENOVA

Associates, inc. STRUCTURAL ENGINEERING + DESIGN
 1212 Germantown Pike, Suite 5
 610.270.9511Phone

 Plymouth Meeting, PA 19462
 610.270.9657 Fax



^{1.} PROVIDE PRE-CAST HEADER FOR ALL DOORS IN ELEV. SHAFT; COORD. LOC. AND QTY. WITH ARCH'L DWGS.



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